W-16065

APT ANALYSIS

SITE: Layout I-95 North Lake Blvd

Section 19 Township 42 S Range 43 E

REPORT: _____

GEOLOGIC DATA: pg. ____, _____

WELL NUMBER OF WELL DESCRIBED: _____

DEPTH (LSD)	LITHOLOGY

Producing zone interval: 20 - 320 (lsd) _____ (msl)

Aquifer name: _____

Static Water Level at the site is approximately _____ ft. msl.

WELL DESCRIPTIONS:

Well	Diam. (in)	Total Depth	Cased Depth	Scr/Open Interval	Slot Size	Radius
PW	6	196	40	40-196		
OBD1	2	160	150	150-160		
OBS-1	2	35	30	30-35		
OBD-2	2	160	150	150-160		
OBS-2	2	35	30	30-35		
OBD-3	2	160	150	150-160		
OBS-3	2	35	30	30-35		
OBM-1	2	35	30	30-35		

INFLUENCING FACTORS:

APT: pg. 1

Started: 07/20/1987 2348

Duration: 4090 mn

Discharge: 170 gpm

Recovery: 784 mn

Comments:

1) _____

_____

2) _____

_____

3) _____

_____

CONSULTANT'S ANALYSIS: pg. _____

Method: Jacob

Results:

Well	Transmissivity (GPD/FT)	S or Sy	Leakance ( )
D1	8854	.00017	
D2	11510	.00004	
D3	13703		

Comments: _____

Method: Walton

Results:

Well	Transmissivity (GPD/FT)	S or Sy	Leakance ( )
D2	7807	.00013	
D3	8614	.000087	
D			

Comments: _____

Method: _____

Results:

Well	Transmissivity (GPD/FT)	S or Sy	Leakance ( )

APT ANALYSIS

SITE: I-95 near North Lake Blvd.
Section 19, Township 42S, Range 43E

REPORT:

GEOLOGIC DATA:

Based on the drilling logs, well cuttings, and geophysical logs, the site lithology may be summarized as follows:

0'-40'	Sand and Shell; medium to coarse sand with fragmented to whole shell.
50'-80'	Shell and Sand; as above.
80'-114'	Sand and Shell; fine to coarse sand with shell fragments.
114'-132'	Limestone and Shell; slightly to very porous, vugular porosity 114'-120', shell fragments.
132'-170'	Limestone; low to high porosity (vugular porosity 136-140).
170'-200'	Limestone and Shell; low to moderate porosity, worn shell fragments, moderate to good permeability.
200'-240'	Clay, Limestone, Shell; sandy, grayish, olive green clay, very low permeability.
240'-300'	Limestone, Shell, and Sand; low to moderate porosity, worn shell fragments, medium to coarse sand, moderate permeability.
300'-360'	Clay, Shell, Limestone, Sand; olive brown clay, content increasing with depth, very low permeability.

There is no zone of secondary permeability at the site although zone with vugular porosity were noted at 114 and 136 feet, is ____ ft. thick with its bottom at ~~the aquifer~~ ____ ft. NGVD.

the aquifer

There is a significant clay layer from 200 to 240 ft.

WELL DESCRIPTIONS:

Well	Diam. (in)	Total Depth	Cased Depth	Screen /Open	r (ft)	Elev. TOC (ft NGVD)
PW-1	6	~~320~~ 196	40	SCREEN		15.59
S-1	2	45	40	SCREEN	30	13.95
D-1	2	160	150	SCREEN	31	13.79
S-2	2	45	40	SCREEN	91	13.97
D-2	2	160	150	SCREEN	91	13.79
S-3	2	45	40	SCREEN	201	13.72
D-3	2	160	150	SCREEN	201	14.50
M-1	2	45	40	SCREEN	>1500	13.42

INFLUENCING FACTORS:

APT:

Started: 2,348 hrs. 7/20/87
Duration: 4,090 minutes
Discharge: 170 GPM measured with 6" manometer tube and 3" orifice
Recovery: 784 minutes

Comments:

1. The flow rate was much lower than anticipated and because drawdown in the production well was near maximum for the 6" centrifugal pump, the pump was extremely sensitive to valve position. Flow varied 8% during the test ranging from 164 to 174 GPM. Changes were gradual and corrected by adjusting the valve when they became significant. Average discharge was ____ GPM.

2. There was .03" of rain 7/21 from 1256 to 1301.

3. Water levels were measured continuously with an In-Situa 2000 hydrologic analysis system.

CONSULTANT'S ANALYSIS:

REANALYSIS:

RECOMMENDED VALUES:

REFERENCES:

Well	T (FT²/DAY)	S	K'/b' (day⁻¹)
D2	1000	.00020	.005
D3	930	.00014	.004

Comments:

1) A good type curve match could not be obtained for well D1A.
2) The matches for wells D2 and D-3 were good where data after approximately 5 minutes into the test was used.
3) The transmissivities based on this method seem extremely low. ~~(Check assumptions using Hantush ??)~~ A check on ~~minimum on~~ the ~~Not~~ method's time limits using the analysis results show that the system at I-95 most likely violates the method's assumptions leaving the results highly questionable.

Method: Theim

Results:

Wells	T (FT²/DAY)
D-1, D-2	2600
~~BE2~~ D-1, D-3	2200
D-2, D-3	1800

Comments:

4. Drawdowns in the deep observation well were largest ranging from 11 feet in D-1 to 6.4 feet in D-3. No significant drawdowns were observed in the shallow wells.

5. The recovery data showed a net water level decline of approximately .5 feet in all wells. This is attributed to background water level changes, not to the pump test.

ANALYSIS:

Method: Jacob
Results: None, method assumptions not met.
Comments:
1) A good straight line fit was not possible for wells D-1 or D-2. A good fit was obtained with the data from D-3A, but the analysis showed that the criteria that $u < .01$ was violated so the results were not valid.

Method: Theis
Results: None, no decent type curve matches.

Method: Hantush-Jacob
Results:

Method: Theis Recovery
Results: None, method assumptions not met.

Comments:

1) There was no good straight line fit to the data for well D-1.
2) Straight lines were matched to the data for wells D-2, and D-3, but the analysis criteria that $u < .01$ was violated so the results were not valid.

The results from this pump test are extremely inconsistent. No confinement was noted in either the drilling logs or the cuttings ~~descs~~ descriptions, but no drawdown was seen in the shallow wells despite a ~~10 ft~~ 60-ft head gradient between the shallow and deep observation wells. The well is supposedly screened between 40 [Check well geophys. logs]

see memo to file

Theim Analysis of I-95 APT Data (see K&D pg. 47)

$$T = \frac{Q}{2\pi(s_1 - s_2)} \ln\left(\frac{r_2}{r_1}\right) \qquad Q = 170 \text{ GPM}$$

Using last measured data:

A. D-1, D-2

$s_{D1} = 11.02$ $r_{D1} = 30$
$s_{D2} = 8.79$ $r_{D2} = 90$

$T =$ 19200 GPD/FT
2600 FT^2/DAY

B. D-1, D-3

$s_{D1} = 11.02$ $r_{D1} = 30$
$s_{D3} = 6.45$ $r_{D3} = 200$

$T =$ 16200 GPD/FT
2200 FT^2/DAY

C. D-2, D-3

$s_{D2} = 8.79$ $r_{D2} = 90$
$s_{D3} = 6.45$ $r_{D3} = 200$

$T =$ 13300 GPD/FT
1800 FT^2/DAY

System is probably behaving as leaky, not confined ???
If system were behaving as leaky, would T increase w/ more distant wells. If partial penetration were affecting results again — should increase with more distant wells.

APT ANALYSIS

SITE: I-95, near North Lake Blvd.
Section 19, Township 42S, Range 43E
elevation: 13' NGVD

REPORT: None

GEOLOGIC DATA:
Based on drilling logs, well cuttings, and geophysical logs the site lithology may be summarized as follows:

0'-40'	Sand and shell; medium to coarse sand with shell, fragmented and whole.
40'-55'	Sand and fine shell fragments; trace of limestone and lime mud.
55'-115'	Sand and fine shell fragments; high percentage of clay and lime mud with trace of poorly lithified limestone.
115'-132'	Limestone and Shell; slightly to very porous, vugular porosity 114'-120' possibly filled with fines insitue, shell fragments.
132'-170'	Limestone; low to high porosity (vugular porosity 136'-140' probably filled with fines insitue).
170'-200'	Limestone and Shell; low to moderate porosity, worn shell fragments, possibly moderate to good permeability.
200'-240'	Clay, Limestone and Shell; sandy, grayish, olive green clay, very low permeability.
240'-300'	Limestone, Shell and Sand; low to moderate porosity, worn shell fragments, medium to coarse sand, moderate permeability.
300'-360'	Clay, Shell, Limestone and Sand; olive brown clay, content increasing with depth.

Sample descriptions corresponding to depths 0'to 115' were based on samples taken during the drilling of intermediate well #1. It is suspected most of the clay present in the formations encountered was dissolved in the circulation fluids while drilling all the other wells.

There is no zone of secondary permeability at the site although zones with vugular porosity were noted at 115 and 136 feet. The aquifer is 270 ft thick with the bottom occurring at -257 ft NGVD.

The aquifer at the site is semi-confined with a very low leakance. The confining layer occurs from 55 to 115 feet below land surface.

WELL DESCRIPTIONS:

Well	Diam. (in)	Total Depth	Cased Depth	Screen /Open	r (ft.)	Elev. TOC (ft. NGVD)
PW-1	6	196	120	Screen		15.59
S-1	2	45	40	Screen	30	13.95
D-1	2	160	150	Screen	31	13.79
S-2	2	45	40	Screen	91	13.97
D-2	2	160	150	Screen	91	13.79
S-3	2	45	40	Screen	201	13.72
D-3	2	164	154	Screen	201	14.5
M-1	2	45	40	Screen	>1500	13.42
I-1	2	118	113	Screen	30	unk.

APT #1:

Started: 2348 hours, 7/20/87
Duration: 4090 minutes
Discharge: 170 GPM, measured with 6" manometer tube and 3" orifice. Discharge was to a canal > 1000 feet to the south.
Recovery: 784 minutes

Influencing factors:

1. Drawdown in the production well was large enough to significantly affect the performance of the 6" centrifical pump. Very early time drawdowns are not reliable due to the rapid drop in pumping rate as the pump tried to overcome the increases in well head vacuum.

2. Flow varied at least 8% during the test and were observed to range from 165 to 178 GPM. Observed changes were gradual, and attempts to stabilize flow rates were made periodically by adjusting the discharge valve as changes became significant.

3. The site was located next to Interstate 95. Increases in early morning and early evening traffic may be responsible for fluctuations in the water levels of the shallow wells.

4. There was .03" of rain 7/21 from 1256 to 1301.

Comments:

1. Flow rates were much lower than expected.

2. Flow rates were monitored and recorded every hour during the duration of the drawdown portion of the test.

3. Stress on the pump due to the high well-head vacuum made the

pump very sensitive to changes in the position of the discharge valve.

4. Water levels were measured continuously using down-hole pressure transducers connected to a In-Situ 2000 Hydrologic Analysis System. Water levels were also checked every two hours using a hand-held, chalked tape.

5. Minor fluctuations in the local water table were monitored by hand taping the water levels in MW-1 every two hours.

ANALYSIS:

Data analysis for this test were inconclusive due to the effects of early-time variations combined with the storage and leakance effects of the thick aquitard sequence.

APT #2

Started: 1402, 7/12/87
Duration: 212 minutes
Discharge: 218 GPM, measured with a 6" diameter manometer tube with a 3" orifice plate. Discharge was to a ditch located > 1000 feet to the south.

Recovery: 125 minutes.

Influencing factors:

1. The In-Situe Hydrologic Analysis Unit was started approximately .05 minutes after the pump was started.

Comments:

1. A submersible pump with an intake set at 63 feet below TOC was used for this test.

2. Flow rates were very steady all the way through the test and varying less than 2 %.

ANALYSIS:

Jacob: Neither drawdown nor recovery data met the restrictions of $u < .01$

APT #3

APT #3 was a very short pump test run using an almost identical pumping rate as was used in APT #2. Drawdown curves showed no significant variation from APT #2 drawdown curves.

Walton:	Wells	T	S	K'/b'
	D-2	7,800 GPD/FT (1,040 sqFT/D)	1.3x10-4	0.032 Day-1
	D-3	8,600 GPD/FT (1,150 sqFT/D)	8.7x10-5	0.009 Day-1

Average values: T=8,200 GPD/FT (1,096 sqFT/D, S=2x10-4

Calculated values increased with increasing r. This is probably due to the effects of water coming from storage in the aquitard.

Drawdowns in the wells did not reach equilibrium due to the short duration of the pump test.

REFERENCES: Walton, W. C. 1979

I-95 APT #2

Walton : Leaky aquifer w/ no release of H_2O from storage in aquitard

$Q = 218$ GPM

Well 3D, $r = 201'$

$W(u, r/B) = 1$, $1/u = 1$, $s = 2.9'$, $t = 1.1$ min

$r/B = 0.33$, $b' \approx 60'$

$$T = \frac{114.6(Q)W(u\,r/B)}{s} = \frac{114.6(218\,GPM)1}{2.9\,ft} = 8{,}614\ GPD/F$$

$$S = \frac{T(t)u}{1.87(r^2)} = \frac{8614\ GPD/Ft(1.1\,min)(1)}{1.87(40{,}401\ Ft^2)(1440\ min/day)} = 8.7 \times 10^{-5}$$

$$L = (r/B)T/r^2 = (0.33(8{,}614\ GPD/Ft)/40{,}401\ Ft^2)/7.48$$

$L = .009\ Day^{-1}$ $K' = .56\ Ft/Day$

Well 2D, $r = 91$ $Q = 218$ GPM

$W(u, r/B) = 1$, $1/u = 1$, $s = 3.2'$, $t = .38$ min, $r/B = 0.25$

$b' = 60'$

$$T = \frac{114.6(218\,GPM)1}{3.2\,Ft} = 7807\ GPD/Ft$$

$$S = \frac{7807\ GPD/Ft(.38\,min)1}{1.87(8281\ Ft^2)(1440\ min/day)} = 1.3 \times 10^{-4}$$

$$L = \frac{0.25(7807\ GPD/Ft)}{8281\ Ft^2(7.48\ G/Ft^3)} = .032\ Day^{-1}$$

Drilling Log
I-95 Test Hole

0-4 fine sand with iron stain
4-15 clean sand, colorless
15-20 sand with lots of whole and fragmented shells
20-30 sand with shell fragments (25-35 formation taking fluid)
35 increase in "sand content in desander, increade in fine shell
41-50 shell hash with small amount of sand
51-61 shell hash with sand, shell is dark
72 began getting larger shell with beach pebbles
80-90 fine sand with silt and shell
100-100 as above
114-120 limestone fragments
125-132 bit chatter
136 drilling slowed
140-150 white limestone (bit chatter at 140)
150 limestone, micrite, hard
151-161 alternating hard and soft white limestone with a small amount of shell
160-170 limestone, white and grey, granular
198-202 sandy green clay
202-221 sandy green clay with limestone stringers
221-241 sandy green clay with limestone stringers, clay decreasing and limestone increasing with depth
261 limestone with silt and clay
261-281 poorly consolidated silty limestone with shell, light grey
281-301 very soft limestone with silt
301-321 as above but with more clay
321-341 as above with still more clay and shell
341-361 lots of clay with some limestone

I-95 Test Hole I-95TH

0-4 fine sand, light brown with iron stain
4-10 fine sand, light brown
10-15 fine sand, light brown
15-20 shell and fine sand, light brown
20-30 fine sand
30-40 medium sand with shell fragments
40-50 sand, light brown, with shell fragments
50-60 sand, darker with increasing shell fragments
60-72 sand, shell fragments
72-80 sand, beach pebbles,shell
80-90 sand,shell (poor returns)
90-100 sand, shell (poor returns)
100-114 sand, shell (poor returns)
114-120 coral, rock, sand,shell
120-132 shell fragments, sand (poor returns)
132-136 limestone, cream and dark
136-140 limestone, cream with shell (slow penetration)
140-150 limestone, cream with shell
150-160 as above
160-170 as above
170-180 limestone, cream and grey with shell
180-190 shell and limestone, light brown to grey, easier drilling
190-200 as above
200-202 shell, limestone, and clay
202-210 sandy green clay, shell, limestone
210-220 as above
220-230 limestone, cream, clay, shell
230-240 grey limestone, green clay, shell
240-250 limestone,grey, clay, shell
260-270 poorly consolidated, silty limestone
270-280 as above
280-290 soft poorly consololidated limestone
290-300 as above
300-310 soft poorly consolidated limestone, clay
310-320 as above
320-330 clay, limestone
330-340 clay, limestone
340-360 green clay, limestone, shell

Table of Results

Site #	Site ID	PUMPED WELL #	OBSERV. WELL #	PROD Cased /TD	OBS. CASED / TD	Well RADIUS r (FT)	PUMP RATE (GPM)	HOURS PUMPED	MAXIMUM DRAWDOWN	TRANSMISS (GPD/FT)	STORAGE	LEAKANCE (GPD/FT3)	ANALYSIS METHO
1	Milkitary Trail	PW	D-1	20/250	170/180	30	189	30.5	0.8	157000	.0002		Neuman Fully Penetrating
			D-1		" "	"			"	237000	.0003		Jacob Drawdown
			D-2		170/180	90			0.6	188000	.0002		Neuman
			D-2		" "	"			"	227000	.0007		Jacob Drawdown
			D-3		170/180	200			0.4	103184	.00013		Neuman
			D-3		" "	"			"	269000	.0007		Jacob Drawdown
			S-1		40/45	30			1.4	n/a	n/a		--
			S-2		40/45	90			0.75	40000	.00063		Neuman
			S-3		40/45	200			0.4	n/a	n/a		--

Note: Generator failure caused premature termination of Drawdown cycle and caused recovery measurements to be delayed 20 minutes. Also, generator failure caused additional 15 minute gap in data collection during recovery.
Also, S-1 and S-3 were not analysed for some reason, probably was not considered a priority to analyse all 3 shallow wells, one (S-2) was all that was analysed.

S-2, S-3 Recovery plots do not follow a stright line fit, therefore recovery method was not used to compute parameters.

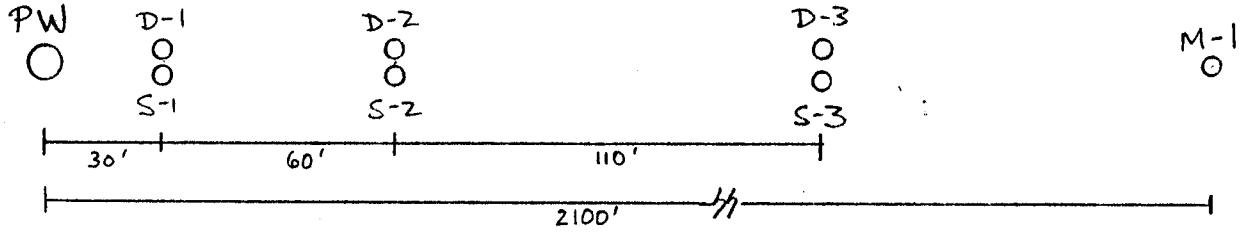
2	I-95 & Northlake Blvd.	PW-1	D-1	10/196	150/160	31	218	3.5	14.0	8854	.00017		Jacob
			D-2		150/160	91			10.5	7807	.00013		Walton
			D-2		" "	"			"	11510	.00004		Jacob
			D-3		170/180	200			7.6	8614	.000087		Walton
			D-3		" "	"			"	13703	--		Jacob

Note: See file for good summary of events by Don P. Basically, three APTs were run, the second of which was used for analysis because the Q did not fluctuate during the test. The Jacob method did not meet the limiting conditions for u<.01. The first test was run for 33 hours before the pump failed, Q was variable and uninterprestable. Confidence in the final parmater estimation was low due to the inconsistency evident beytween analysis types.

USGS stuff: Only two sites in study area by USGS. These test results were highly suspect due to multiple zone completed wells. M.J. did not include these in her report and I just assume not include them in mine.

I-95 Site

Site Layout -



Well Descriptions:

Well	Type	Diam. (in)	* Total Depth	* Cased Depth	* Screened Interval
PW	Pumping	6	320'	20'	20'-320'
D-1	Observation	2	160'	150'	150'-160'
S-1	"	2	35'	30'	30'-35'
D-2	"	2	160'	150'	150'-160'
S-2	"	2	35'	30'	30'-35'
D-3	"	2	160'	150'	150'-160'
S-3	"	2	35'	30'	30'-35'
M-1	"	2	35'	30'	30'-35'

Obse

* Well depths are approximations based on presently available data. The productio well will be drilled to the base of the Surficial Aquifer. The depths of the observation wells will depend on the layering found when the pumping well is drille

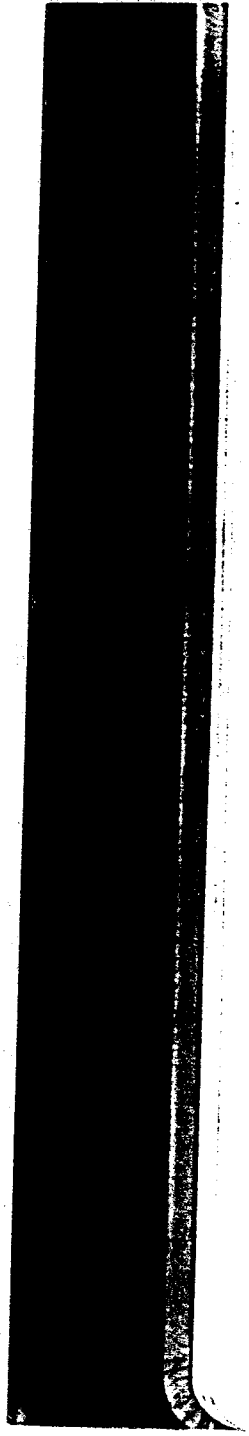
Well Construction - The wells will have PVC casing and PVC slotted screen. The will be a 2" gravel pack around the screen and 2" of ceme grout around the casing.

wells will located on the I-95 right-of-way as far from the road edge as p

Palm Beach County Aquifer Performance Tests

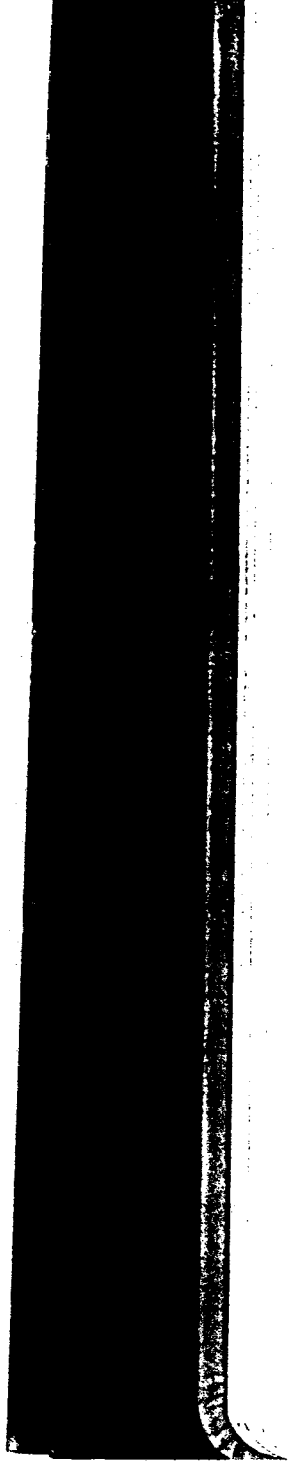
Table of Information

MAP NO.	LAND OWNER	SOURCE	Reanalyzed	DATE OF TEST	S/T/R	PlanarX	PlanarY	Lith.Well
1	DOT Miltary-Don.Ross	SFWMD	Y	12/15/87	24/41/42	792200	932410	PW
2	DOT I-95-Northlake	SFWMD	Y	07/12/87	19/42/43	with techs		PW-1



LEVELS TO ESTABLISH ELEVATIONS ON PVC GR
WATER MONITORING WELLS LOCATED @ I-95
LOCATED ON THE EASTERLY R/W OF I-95 ± 19
SOUTH OF NORTH LAKE BOULEVARD

STA	+	H.I.	−	ELEV.
B.M.				14.97
	4.20	19.17		
	3.07			
T.B.M.			3.58	15.59
			3.69	
T.B.M.			5.22	13.95
			2.05	
T.B.M.			5.38	13.79
			1.89	
T.B.M.			5.20	13.97
			2.07	
T.B.M.			5.38	13.79
			1.89	
T.B.M.			5.45	13.72
			1.82	
T.B.M.			4.67	14.50
			2.60	



STA.	+	H. I.	−	ELEV.
		19.17		
TP			3.85	15.32
			3.42	
	4.70	20.02		
	2.57			
TP			3.97	16.05
			3.30	
	4.01	20.06		
	3.26			
TP			4.68	15.38
			2.59	
	4.84	20.22		
	2.43			
TP			5.29	14.93
			1.98	
	4.02	18.95		
	3.25			
T.B.M.			5.53	13.42
			1.74	
	5.39	18.81		
	1.88			
TP			3.88	14.93
			3.39	
	5.07	20.00		
	2.20			

TE

BENCH MARK "I 95 - 1" DOUBLE SPIKE IN FENCE POST
AS PER PG. 34

TOP OF 6" PVC GROUND WATER MONITORING WELL "P

TOP OF 2" PVC GROUND WATER MONITORING WELL "S-

TOP OF 2" PVC GROUND WATER MONITORING WELL "D-

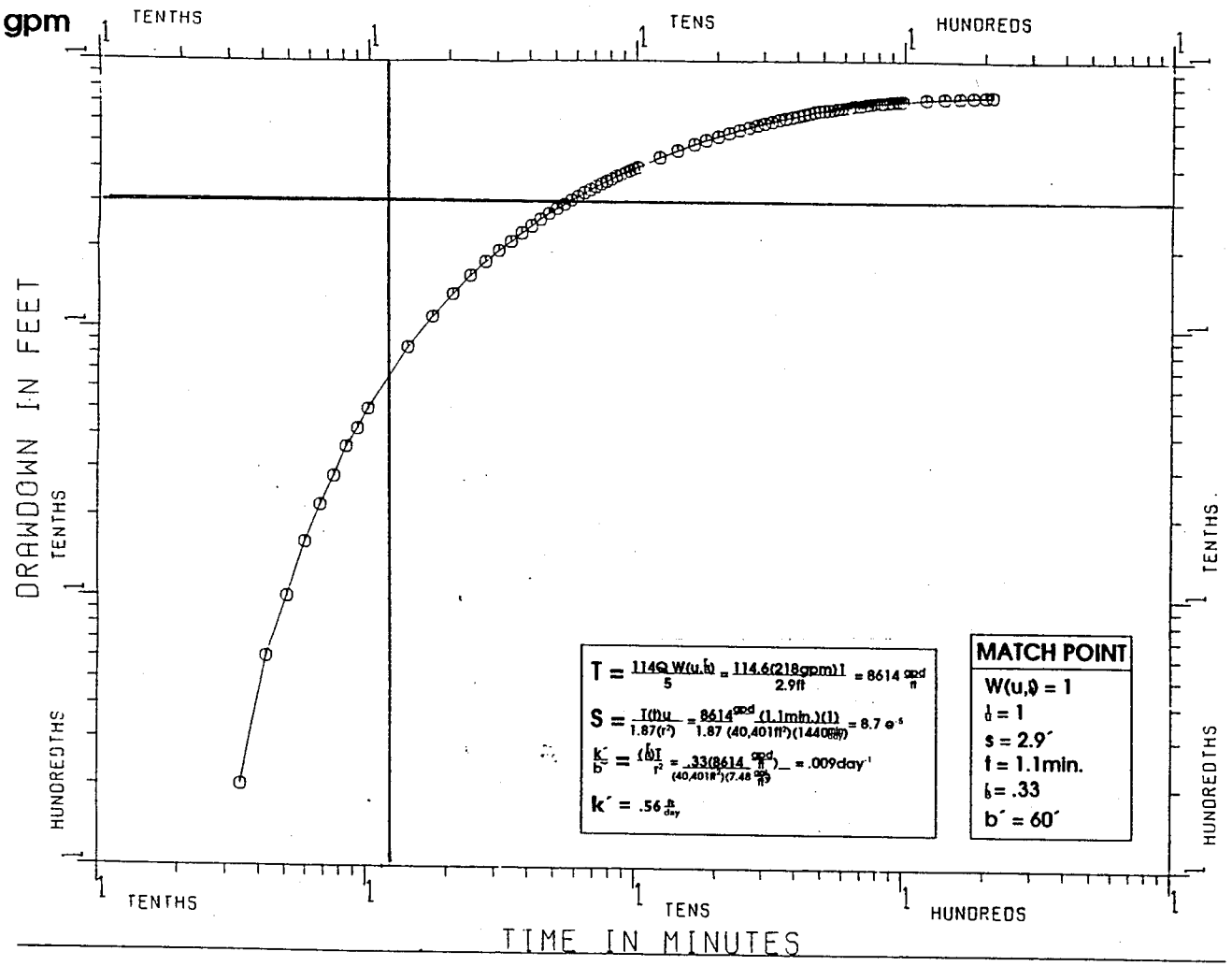
TOP OF 2" PVC GROUND WATER MONITORING WELL "S-2

TOP OF 2" PVC GROUND WATER MONITORING WELL "D-2

TOP OF 2" PVC GROUND WATER MONITORING WELL "S-3

TOP OF 2" PVC GROUND WATER MONITORING WELL "D-3

I-95 & North Lake Blvd. Site
Walton Analysis
OBSERVATION WELL: 3D
r = 201.0 ft Q = 218.0 gpm



I-95 PT DRAWDOWN

I95APT.DWG

I–95 SHORT DURATION APT

D_1 : R = 31' Q = 218 GPM

D_2 : R = 91' Q = 218 GPM

D_3 : R = 201' Q = 218 GPM

D_1 : $\Delta S = 6.5$, $t_{01} = .09$ min.

$$T_1 = \frac{264(218)}{6.5} = 8{,}854 \text{ GPD/FT} = 1184 \text{ Ft}^2\text{/day}$$

$$S_1 = \frac{2.25(1184 \text{ ft}^2\text{/day})(.09 \text{ min})}{(31 \text{ ft})^2(1440 \text{ min/day})} = .00017$$

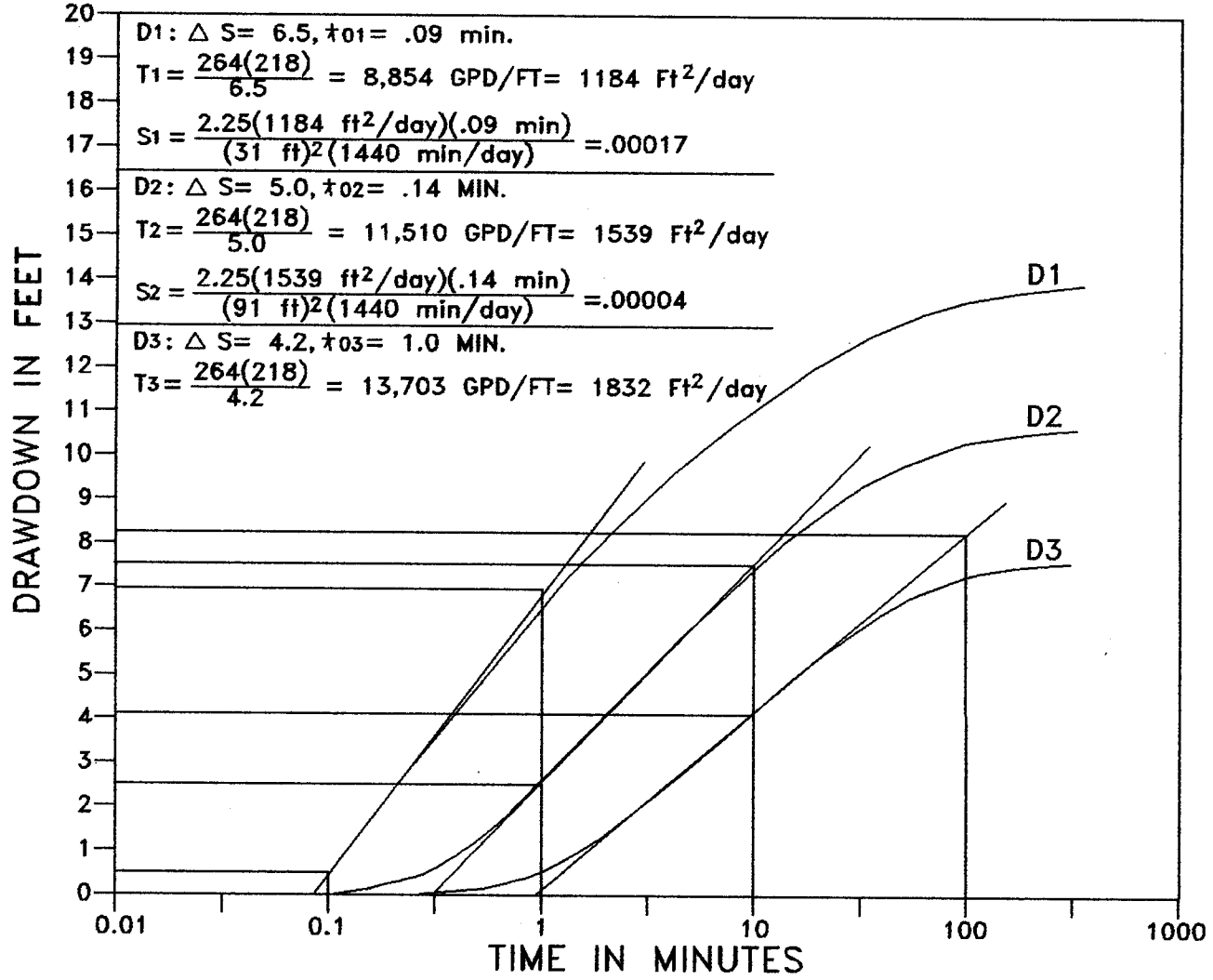
D_2 : $\Delta S = 5.0$, $t_{02} = .14$ MIN.

$$T_2 = \frac{264(218)}{5.0} = 11{,}510 \text{ GPD/FT} = 1539 \text{ Ft}^2\text{/day}$$

$$S_2 = \frac{2.25(1539 \text{ ft}^2\text{/day})(.14 \text{ min})}{(91 \text{ ft})^2(1440 \text{ min/day})} = .00004$$

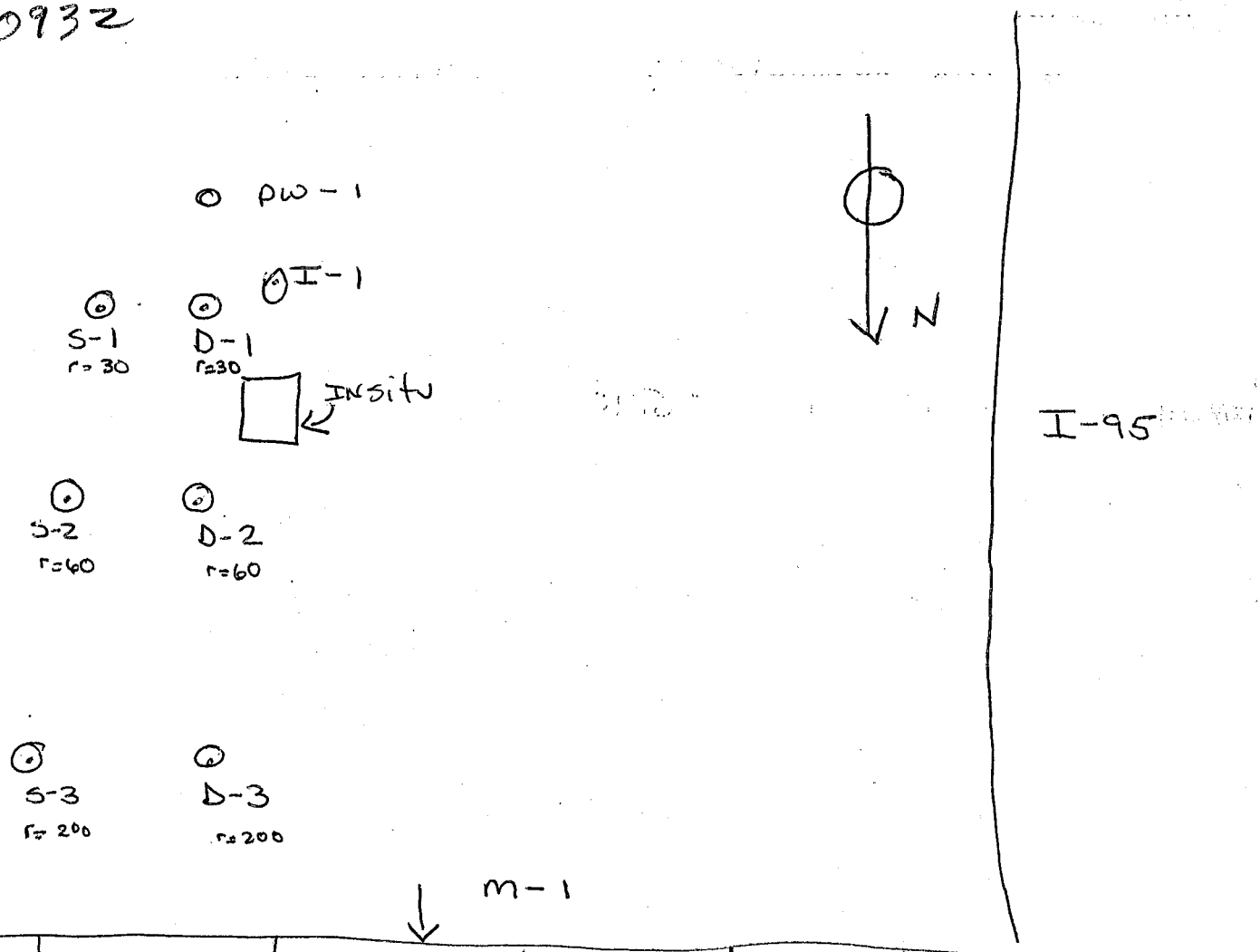
D_3 : $\Delta S = 4.2$, $t_{03} = 1.0$ MIN.

$$T_3 = \frac{264(218)}{4.2} = 13{,}703 \text{ GPD/FT} = 1832 \text{ Ft}^2\text{/day}$$



Don phone
588-0932

Site Diagram



stat	Well #	Transducer	Scale Factor	Input #	Depth	* Recovery
O	S-1	599	10.06	1 30	12'	Mode In-situ 1730
O	S-1A	2247	10.12	2 30	17'	Aborted pump
O	D-1	718	10.08	3 31	25'	on, Draw Down
O	I-1	710	10.04	4 30	25'	Re-started
O	I-1A	158	49.82	5 30	27'	(Insitu). Successful
O	S-2	209	10.02	6 91	12'	Recovery
O	D-2	2615	10.08	7 91	25'	Achieved 1759
O	S-3	1993	10.11	8 201	15'	pump off
O	D-3	2157	10.06	9 201	20'	Run 1 - DD
						12/7/87
	Hermit					1231 - 1730
	M-1	137	49.76	1	10	Run 2 - DD & Recover
						1749 - end
						Max DD
						17 30 → 1759.01

Fast Data D-2 ✓

Scale Factor 1 ? Pumpage 216

AQUIFER TEST DATA

Owner _____ Address I-95 County Palm Beach State _____

Date _____ Company performing test _____ Measured by _____

Well No Manometer Distance from pumping well _____ Type of test _____ Test No. _____

Measuring equipment 6" Dia manometer w/ " orifice plate

Time Data

Pump on: Date _____ Time _____ (t_0)
Pump off: Date _____ Time _____ (t'_0)
Duration of aquifer test:
Pumping _____ Recovery _____

Water Level Data

Static water level _____
Measuring point _____
Elevation of measuring point _____

Comments on factors affecting test data

Date	Clock time	Time since pump started t	Time since pump stopped t'	t/t'		held @	wet to	Water level measurement	Correction or Conversion	Water level	Water level change s or s'			
~~12/7~~	~~0915~~					~~6.00~~	~~825~~	5.175						

I95 APT Run # 2
Drawdown

I-95 PT
Run 1
12/07/87

SE200B DATA
constant rate test

TRANSDUCER TABLE

Input 1: S-1
  Transducer s/n: 599
  Scale factor: 10.06

Input 2: S-1A
  Transducer s/n: 2247
  Scale factor: 10.12

Input 3: D-1
  Transducer s/n: 718
  Scale factor: 10.08

Input 4: I-1
  Transducer s/n: 710
  Scale factor: 10.04

Input 5: I-1A
  Transducer s/n: 158
  Scale factor: 49.82

Input 6: S-2
  Transducer s/n: 209
  Scale factor: 10.02

Input 7: D-2
  Transducer s/n: 2615
  Scale factor: 10.08

FAST DATA

Input 8: S-3
  Transducer s/n: 1993
  Scale factor: 10.11

Input 9: D-3
  Transducer s/n: 2157
  Scale factor: 10.06

PUMP SCHEDULE

Drawdown for 360 min
Pump at 250 GPM
Pump set at 999.01 feet

Recovery for 240 min

SAMPLING SCHEDULE

0-10	sec	@	1 sec
10-60	sec	@	5 sec
1-10	min	@	20 sec
10-100	min	@	2 min
100-1000	min	@	20 min
1000-10000	min	@	60 min
10000-99999	min	@	200 min