UNITED STATES UNITED STATES DEPARTMENT OF THE INTERIOR
GEOLOGICAL SURVEY GEOLOGICAL SURVEY

HYDROGEOLOGIC DATA FOR THE BEAR CREEK SUBSURFACE-HYDROGEOLOGIC DATA FOR THE BEAR CREEK SUBSURFACE-INJECTION TEST SITE, ST. PETERSBURG, FLORIDA
By J. J. Hickey and G. L. Barr By J. J. Hickey and G. L. Barr

Open-File Report 78-853 Open-File Report 78-853

Prepared in cooperation with the Prepared in cooperation with the
CITY of ST. PETERSBURG, FLORIDA

Tallahassee, Florida Tallahassee, Florida 1979

1979

UNITED STATES DEPARTMENT OF THE INTERIOR
CECIL D. ANDRUS, Secretary

 \bar{r}

CECIL D. ANDRUS, Secretary

GEOLOGICAL SURVEY

H. William Menard, Director SURVEY H. William Menard, Director

For additional information write to: For information write to:

U.S. Geological Survey U.S. Survey Water Resources Division Water Resources Division 325 John Knox Road, Suite F-240 325 John Knox Road, Suite F-240 Tallahassee, Florida 32303 Tallahassee, Florida 32303

CONTENTS CONTENTS

 $\begin{aligned} \frac{d\mathbf{r}}{d\mathbf{r}} &= \frac{d\mathbf{r}}{d\mathbf{r}}\,,\\ \frac{d\mathbf{r}}{d\mathbf{r}} &= \frac{d\mathbf{r}}{d\mathbf{r}}\,, \end{aligned}$

ILLUSTRATIONS ILLUSTRATIONS

Page Page

TABLES TABLES

 $\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^{2}=\frac{1}{2}\left(\frac{1}{2}\right)^{2}$

 \mathcal{A}^{\prime}

 \sim

 $\label{eq:2} \frac{1}{2} \sum_{i=1}^n \frac{1}{2} \sum_{j=1}^n \frac{1}{$

Factors for converting U.S. inch-pound units shown to four significant figures. to metric units are Factors for converting U.S. inch-pound units to metric units are shown to four significant figures.

 \sim

HYDROGEOLOGIC DATA FOR THE BEAR CREEK SUBSURFACE-HYDROGEOLOGIC DATA FOR THE BEAR CREEK SUBSURFACE-INJECTION TEST SITE, ST. PETERSBURG, FLORIDA INJECTION TEST SITE, ST. PETERSBURG, FLORIDA

By J. J. Hickey and G. L. Barr
ABSTRACT

ABSTRACT

Lithologic, hydraulic, geophysical, and water-quality data were col-Lithologic, hydraulic, geophysical, and water-quality data were collected at the Bear Creek subsurface-injection test site. The data will assist in evaluating the feasibility of subsurface injection of storm run-lected at the Bear Creek subsurface-injection test site. The data will assist in evaluating the feasibility of subsurface injection of storm runoff. off.

An exploratory hole and five observation wells were constructed at An exploratory hole and five observation wells were constructed at this site between October 1974 and April 1976. The exploratory hole, this site between October 1974 and April 1976. The exploratory hole, drilled to 1,290 feet below land surface, was the second exploratory hole drilled to 1,290 feet below land surface, was the second exploratory hole
drilled at the test site. The first, 540 feet distant, was 3,504 feet deep. The observation wells constructed within the second exploratory deep. The observation wells constructed within the second exploratory hole ranged in depth between 340 to 1,267 feet. hole ranged in depth between 340 to 1,267 feet.

The lithology of the upper 185 feet at the test site is predominant-The lithology of the upper 185 feet at the test site is predominantly sand and marl. From 185 to 3,504 feet, limestone and dolomite predom-ly sand and marl. From 185 to 3,504 feet, limestone and dolomite predominate. Below 1,290 feet, gypsum is also present. inate. Below 1,290 feet, gypsum is also present.

Vertical intrinsic permeability of cores extracted dyring the drill_{$\frac{1}{1}$ 4 ing of the second exploratory hole ranges from 1.20 x 10 to 9.87 x 10} centimeters squared. Porosity of the cores ranges from $0₅$ to 39.5 per₇ cent. Compressibility of the cores ranges from 1.2×10^{-7} to 1.5×10^{-7} square inches per pound. square inches per pound. Vertical intrinsic permeability of cores extracted during the drill $\frac{1}{14}$ centimeters squared. Porosity of the cores ranges from $0_{\epsilon}5$ to 39.5 per $_{7}$ cent. Compressibility of the cores ranges from 1.2 \times 10 $^{\circ}$ to 1.5 \times 10

A 73-hour withdrawal test was run in the test injection well. Water was pumped at a rate of 3,450 gallons per minute. At the site, chloride concentration in water from 192 to 340 feet ranged from 150 to 680 milligrams per liter, and from 500 to 1,267 feet ranged from 16,000 to 20,000 grams per liter, and from 500 to 1,267 feet ranged from 16,000 to 20,000 milligrams per liter. The chloride concentrations in water from 11 addi-milligrams per liter. The chloride concentrations in water from 11 additional wells near the test site ranged from 72 to 1,100 milligrams per
liter. The wells were 45 to 400 feet deep. liter. The wells were 45 to 400 feet deep. A 73-hour withdrawal test was run in the test injection well. Water was pumped at a rate of 3,450 gallons per minute. At the site, chloride concentration in water from 192 to 340 feet ranged from 150 to 680 milli-

INTRODUCTION INTRODUCTION

The city of St. Petersburg, a municipality within Pinellas County, The city of St. Petersburg, a municipality within Pinellas County, Florida, is experiencing a rapid population growth with increased water-Florida, is experiencing a rapid population growth with increased watersupply demands. Limits have been placed on ground-water withdrawals from supply demands. Limits have been placed on ground-water withdrawals from
the city's well fields which has caused the city to investigate the potential for subsurface storage of storm runoff at the Bear Creek site (fig. tial for subsurface storage of storm runoff at the Bear Creek site (fig. l). The city would like to create a potential water resource for future
non-potable use. non-potable use.

The city of St. Petersburg and the State of Florida Department of The city of St. Petersburg and the State of Florida Department of Natural Resources drilled a test injection well at the Bear Creek site Natural Resources drilled a test injection well at the Bear Creek site
between October 1972 and April 1974 (Black, Crow and Eidsness, 1974). A stream intake structure for the test injection well was completed in 1976. At present (1978), long-term injection of storm runoff has not A stream intake structure for the test injection well was completed in 1976. At present (1978), long-term injection of storm runoff has not been performed. been performed.

Five observation wells, within 540 ft (feet) of the test injection Five observation wells, within 540 ft (feet) of the test injection well, were completed by the city in April 1976. Data collected during well, were completed by the city in April 1976. Data collected during the construction of the observation wells and a subsequent withdrawal the construction of the observation wells and a subsequent withdrawal test on the injection well is the principal subject of this report. test on the injection well is the principal subject of this report.

The U.S. Geological Survey, in cooperation with the city of St. The U.S. Geological Survey, in cooperation with the city of St. Petersburg, is investigating storage of storm runoff in permeable saline Petersburg, is investigating storage of storm runoff in permeable saline water zones within the carbonate rocks that underlie the Bear Creek test water zones within the carbonate rocks that underlie the Bear Creek test injection site (fig. 1). The U.S. Geological Survey's principal interest injection site (fig. 1). The U.S. Geological Survey's principal interest in this investigation is to understand and to document the hydrodynamic in this investigation is to understand and to document the hydrodynamic and chemical behavior of the stored water. and chemical behavior of the stored water.

Purpose and Scope Purpose and Scope

This report presents the hydrogeologic data collected during the This report presents the hydrogeologic data collected during the test drilling and withdrawal testing at the Bear Creek test site. The test drilling and withdrawal testing at the Bear Creek test site. The
data, presented in tables and illustrations, include lithologic descriptions and laboratory analyses of drill cuttings and cores, results of a withdrawal test, hydrographs, geophysical logs, and chemical analyses. tions and laboratory analyses of drill cuttings and cores, results of a withdrawal test, hydrographs, geophysical logs, and chemical analyses.

The data were collected from the Bear Creek site to assist in the evaluation of the following objectives: (1) determine if there are trans-The data were collected from the Bear Creek site to assist in the evaluation of the following objectives: (1) determine if there are transmissive zones which can accept large volumes of storm runoff; (2) determine missive zones which can accept large volumes of storm runoff; (2) determine
the water-quality profile at the site; (3) evaluate effects of well injection on freshwater; and (4) design a long-term monitoring program. These tion on freshwater; and (4) design a long-term monitoring program. These and other determinations will be given in subsequent interpretive reports. and other determinations will be given in subsequent interpretive reports.

To achieve these objectives, an exploratory hole and five observation wells were constructed and a withdrawal test was run on a previously con-To achieve these objectives, an exploratory hole and five observation wells were constructed and a withdrawal test was run on a previously constructed injection test well. Water samples were collected from wells at the test site and also from selected wells near the site. These samples the test site and also from selected wells near the site. These samples were analyzed for water quality. were analyzed for water quality.

Figure 1.--Location of the Bear Creek injection test site, other proposed injection sites, and Tampa Bay
area municipal well fields. area municipal well fields.

Various aspects of the geology and hydrology of Pinellas County have been the subject of several previous investigations. Chen (1965) described Various aspects of the geology and hydrology of Pinellas County have been the subject of several previous investigations. Chen (1965) described the lithologies penetrated from 500 to 5,000 ft below land surface by an the lithologies penetrated from 500 to 5,000 ft below land surface by an oil test hole in Pinellas County as part of his regional stratigraphic oil test hole in Pinellas County as part of his regional stratigraphic analysis of the Paleocene and Eocene rocks of Florida. Hydrologic inves-analysis of the Paleocene and Eocene rocks of Florida. Hydrologic investigations by Heath and Smith (1954), Cherry, Stewart and Mann (1970), and Black, Crow and Eidsness (1970), evaluated the upper carbonate rock sec-tigations by Heath and Smith (1954), Cherry, Stewart and Mann (1970), and Black, Crow and Eidsness (1970), evaluated the upper carbonate rock section to depths of about 400 ft, principally from a water-supply point of view. Greenleaf and Telesca (1975) described the construction of wells at tion to depths of about 400 ft, principally from a water-supply point of
view. Greenleaf and Telesca (1975) described the construction of wells at
the South Cross Bayou test injection site. Black, Crow and Eidsness (1974) investigated the potential for storing storm-water runoff and recovering it from saline zones within the carbonate rocks at the Bear Creek site for investigated the potential for storing storm-water runoff and recovering
it from saline zones within the carbonate rocks at the Bear Creek site for
the city of St. Petersburg, Florida, and the State of Florida Department of Natural Resources. As part of their investigation, an exploratory hole was drilled to a depth of 3,504 ft. They concluded that most of the zones of Natural Resources. As part of their investigation, an exploratory hole
was drilled to a depth of 3,504 ft. They concluded that most of the zones
capable of accepting large volumes of storm-water runoff were above a dept of 1,270 ft. In addition to other tests, two injection tests, each lasting 1 day, were run on a zone between $1,180$ and $1,270$ ft. A transmissibility of 1,270 ft. In addition to other tests, two injection tests, each lasting
1 day, were run on a zone between 1,180 and 1,270 ft. A transmissibility
of 800,000 (gal/d)/ft (transmissivity of 107,000 ft²/d) for the test inj tion zone was reported. Rosenshein and Hickey (1977) discussed the vertical tion zone was reported. Rosenshein and Hickey (1977) discussed the vertical distribution of permeable zones within the carbonate strata underlying the Pinellas peninsula and their potential use for the storage of treated sew-distribution of permeable zones within the carbonate strata underlying the Pinellas peninsula and their potential use for the storage of treated sewage effluent and storm water. Hickey (1977) presented the hydrogeologic age effluent and storm water. Hickey (1977) presented the hydrogeologic data collected at the McKay Creek injection test site (fig. 1). data collected at the McKay Creek injection test site (fig. 1).

Regional Hydrogeologic Setting Regional Hydrogeologic Setting

The Tampa Bay area, including the Bear Creek test site, is underlain by carbonate strata to a depth of about 10,000 ft below land surface The Tampa Bay area, including the Bear Creek test site, is underlain
by carbonate strata to a depth of about 10,000 ft below land surface
(Applin, 1951), except for a thin surficial cover of sand, marl, and clay. The upper 1,300 ft of the carbonate strata is highly transmissive and con-The upper 1,300 ft of the carbonate strata is highly transmissive and constitutes one of the most productive aquifers in the world--the Floridan aquifer. The transmissivity of the aquifer, where it is tapped for water supplies, is estimated to range from 32,000 ft^2/d to more than 270,000 supplies, is estimated to range from 32,000 ft⁻/d to more than 270,000 ft⁻/d (Rosenshein and Hickey, 1977). The aquifer is thought to be made up of permeable zones separated by carbonate strata of low permeability (Rosenshein and Hickey, 1977). The aquifer contains potable water in its entire thickness east and north of Tampa Bay. In general, the flow of potable ground water in the aquifer is toward the Gulf of Mexico and Tampa table ground water in the aquifer is toward the Gulf of Mexico and Tampa Bay. Bay. up of permeable zones separated by carbonate strata of low permeability (Rosenshein and Hickey, 1977). The aquifer contains potable water in its entire thickness east and north of Tampa Bay. In general, the flow of po-

The upper part of the carbonate strata in the Tampa Bay area general-The upper part of the carbonate strata in the Tampa Bay area generally is overlain by less than 200 ft of sand, marl, and clay. The clay com-ly is overlain by less than 200 ft of sand, marl, and clay. The clay commonly forms the basal strata of these surficial deposits and, in northwest monly forms the basal strata of these surficial deposits and, in northwest Hillsborough County, is in part a weathered residue of the underlying car-Hillsborough County, is in part a weathered residue of the underlying carbonate rock. There, according to Sinclair (1974, p. 24–26), the clay has
a vertical hydraulic conductivity of less than 0.003 ft/d. **a** vertical hydraulic conductivity of less than 0.003 ft/d.

4

The surficial sand is generally less than 35 ft thick and during dry weather is generally saturated to within 5 to 10 ft of the land surface. The surficial sand is generally less than 35 ft thick and during dry weather is generally saturated to within 5 to 10 ft of the land surface. During wet weather, the water table in the sand is at or near land sur-During wet weather, the water table in the sand is at or near land surface. The sand in northwest Hillsborough County has a horizontal hydrau-face. The sand in northwest Hillsborough County has a horizontal hydraulic conductivity of 13 ft/d and a vertical hydraulic conductivity in the lic conductivity of 13 ft/d and a vertical hydraulic conductivity in the range of 0.36 ft/d to 13 ft/d (Sinclair, 1974, p. 13).

l,

Summary of Bear Creek Test Site Data Summary of Bear Creek Test Site Data

WELL CONSTRUCTION WELL CONSTRUCTION

Exploratory hole E2 and five observation wells, Bi, B2, B3, B4, and Exploratory hole E2 and five observation wells, Bl, B2, B3, B4, and B5 (table 1), were constructed at the test site between October 1974 and B5 (table 1), were constructed at the test site between October 1974 and April 1976. Already in existence at the site were test injection well Al, April 1976. Already in existence at the site were test injection well Al, observation well A2, and exploratory hole El. Black, Crow and Eidsness observation well A2, and exploratory hole El. Black, Crow and Eidsness (1974) describe these wells. Figure 2 shows the location of the explora-(1974) describe these wells. Figure 2 shows the location of the exploratory holes and wells at the site. tory holes and wells at the site.

Prior Construction at the Test Site

Exploratory hole El, injection well Al, and observation well A2 Exploratory hole El, injection well Al, and observation well A2 (table 1), were constructed about 2 years before the observation wells discussed in this report were drilled. Exploratory hole El (fig. 3), (table 1), were constructed about 2 years before the observation wells discussed in this report were drilled. Exploratory hole El (fig. 3), was drilled to 3,504 ft below land surface. Test injection well Al (fig. was drilled to 3,504 ft below land surface. Test injection well Al (fig. 3) has a 16-in (inch) casing set at 1,016 ft below land surface and is open hole to 1,270 ft. Well A2 (fig. 3) has a 2-in casing and is open between 550 and 570 ft in the annulus of Al. There is also a plugged well (fig. 3) in the annulus of Al with a $1-1/4-$ in galvanized pipe which was to have been open to the interval between 800 and 870 ft. This plugged well has no value and is unnumbered in table 1. well has no value and is unnumbered in table 1. 3) has a 16-in (inch) casing set at 1,016 ft below land surface and is
open hole to 1,270 ft. Well A2 (fig. 3) has a 2-in casing and is open
between 550 and 570 ft in the annulus of A1. There is also a plugged well
(fig.

Exploratory Hole Exploratory Hole

Exploratory hole E2 was drilled during this investigation to a depth of 1,290 ft for preliminary identification of a permeable and semi-confin-Exploratory hole E2 was drilled during this investigation to a depth of 1,290 ft for preliminary identification of a permeable and semi-confining strata at the Bear Creek test site. Location of this well and other wells at the site is shown on figure 2. The upper 500 ft of hole was drilled with cable tool. From 500 to 1,290 ft, the hole was drilled with air-reverse rotary. A 20-in casing was driven and set at 192 ft and a 14- air-reverse rotary. A 20-in casing was driven and set at 192 ft and a 14 in casing was set at 500 ft with its annulus cemented back to 340 ft. From in casing was set at 500 ft with its annulus cemented back to 340 ft. From land surface to 192 ft and 500 to 780 ft, freshwater was periodically added
to the hole to aid drilling. to the hole to aid drilling. ing strata at the Bear Creek test site. Location of this well and other
wells at the site is shown on figure 2. The upper 500 ft of hole was
drilled with cable tool. From 500 to 1,290 ft, the hole was drilled with

During drilling with the cable tool equipment, the bit dropped 2 ft between 318 and 320 ft below land surface, probably because of a cavity During drilling with the cable tool equipment, the bit dropped 2 ft between 318 and 320 ft below land surface, probably because of a cavity in the limestone. in the limestone.

Cores were taken in some strata during the drilling of well E2. Des-Cores were taken in some strata during the drilling of well E2. Descriptions and laboratory analyses of these cores are presented later in criptions and laboratory analyses of these cores are presented later in this report. this report.

 $\sigma_{\rm{max}}$

Figure 2.--Location of exploratory holes and wells at the test site.

 $\overline{}$

 \bar{t}

Figure 3.--Construction diagram of wells A1, A2, B1, B2, B3, B4, B5, E1 and E2.

 Λ

Observation Wells Observation Wells

Five observation wells, Bl, B2, B3, B4, and B5, were constructed in exploratory hole E2 from 192 ft to 1,267 ft below land surface (fig. 3). Five observation wells, Bl, B2, B3, B4, and B5, were constructed in exploratory hole E2 from 192 ft to 1,267 ft below land surface (fig. 3). Exploratory hole E2, drilled to 1,290 ft, was plugged back to 1,267 ft Exploratory hole E2, drilled to 1,290 ft, was plugged back to 1,267 ft with cement. Well Bl has a 4-in casing and monitors the depth interval with cement. Well Bl has a 4-in casing and monitors the depth interval between 1,170 and 1,267 ft. Well B2 has a 2-in casing and monitors the between 1,170 and 1,267 ft. Well B2 has a 2-in casing and monitors the interval between 930 and $1,070$ ft. Well B3 has a 2-in casing and monitors the interval between 750 and 830 ft. Well B4 has a 14-in casing and monitors the interval between 500 and 573 ft. Well B5 is the annular tors the interval between 750 and 830 ft. Well B4 has a 14-in casing and
monitors the interval between 500 and 573 ft. Well B5 is the annular
space between the 14-in and 20-in casings and monitors the interval between 192 and 340 ft. Wells Bl, B2, and B3 have gravel packed intervals with 192 and 340 ft. Wells Bl, B2, and B3 have gravel packed intervals with stainless steel screens and wells B4 and B5 have open-hole intervals. The stainless steel screens and wells B4 and B5 have open-hole intervals. The screen in well B2 became plugged after well construction and had to be per-screen in well B2 became plugged after well construction and had to be perforated. All wells are separated by cement plugs between their screened or open-hole intervals. Wells Bl, B2, B3, B4, and B5 are located 540 ft forated. All wells are separated by cement plugs between their screened or open-hole intervals. Wells Bl, B2, B3, B4, and B5 are located 540 ft from test injection well Al (fig. 2). from test injection well Al (fig. 2).

HYDROGEOLOGIC DATA DATA

The data collected at the Bear Creek test injection site include The data collected at the Bear Creek test injection site include lithologic descriptions of drill cuttings and cores, laboratory core lithologic descriptions of drill cuttings and cores, laboratory core analyses, specific capacity and withdrawal test results, water-level analyses, specific capacity and withdrawal test results, water-level hydrographs, geophysical logs, and water analyses. hydrographs, geophysical logs, and water analyses.

Drill Cuttings and Cores Drill Cuttings and Cores

Drill cuttings were collected every 5 ft from well El and every 10 ft from well E2. These cuttings have been forwarded, as required by state law, to the Florida State Bureau of Geology. The cuttings from well El are described in table 2, except for the interval between 50 to 130 ft which could not be identified. The drill cuttings from well E2 130 ft which could not be identified. The drill cuttings from well E2 describe this interval as follows: 50 to 70 ft, sand; 70 to 110 ft, describe this interval as follows: 50 to 70 ft, sand; 70 to 110 ft, dark gray marl; 110 to 120 ft, dark gray clay; and from 120 to 130 ft, dark gray marl; 110 to 120 ft, dark gray clay; and from 120 to 130 ft, dark gray marl. A graphic lithologic log for well El and depth of cores dark gray marl. A graphic lithologic log for well El and depth of cores from well E2 are shown in figure 4. A graphic lithologic log for well from well E2 are shown in figure 4. A graphic lithologic log for well E2 is shown on the geophysical log illustrations discussed later in this E2 is shown on the geophysical log illustrations discussed later in this report. report. Drill cuttings were collected every 5 ft from well El and every 10
ft from well E2. These cuttings have been forwarded, as required by
state law, to the Florida State Bureau of Geology. The cuttings from
well El are descri

Cores of strata were taken during the drilling of well E2 for the following depth intervals: 720 to 741 ft (5 ft of core recovery), 760 Cores of strata were taken during the drilling of well E2 for the
following depth intervals: 720 to 741 ft (5 ft of core recovery), 760
to 770 ft (1 ft of core recovery), 900 to 950 ft (8.5 ft of core recovery), and 1,140 to 1,150 ft (3 ft of core recovery). Descriptions of ery), and 1,140 to 1,150 ft (3 ft of core recovery). Descriptions of
these cores are given in table 3. Laboratory measurements of the cores

Figure 4.--Lithologic log of well El.

included air and water vertical intrinsic permeability, porosity, inter-included air and water vertical intrinsic permeability, porosity, interval transit time, and compressibility. The results of these measurements val transit time, and compressibility. The results of these measurements are presented in tables 4 through 6. are presented in tables 4 through 6.

Specific Capacity Tests During Drilling of Well E2 Specific Capacity Tests During Drilling of Well E2

Specific capacity tests were run on well E2 while it was being drilled. Specific capacity tests were run on well E2 while it was being drilled, These data can be used to indicate major differences in permeability be-These data can be used to indicate major differences in permeability between intervals of drilled hole, from 192 to 1,290 ft below land surface. tween intervals of drilled hole, from 192 to 1,290 ft below land surface. Table 7 lists the specific capacities in well E2. Table 7 lists the specific capacities in well E2.

Withdrawal Test Withdrawal Test

From May 18, 1976, to May 21, well Al was pumped at a rate of 3,450 From May 18, 1976, to May 21, well Al was pumped at a rate of 3,450 gal/min. During the 73-hour test, water levels were measured in wells gal/min. During the 73-hour test, water levels were measured in wells A2, Bl, B2, B3, B4, and B5. The measurements for A2 are shown in table A2, Bl, B2, B3, B4, and B5. The measurements for A2 are shown in table 8 and for the remainder of the wells in table 9. 8 and for the remainder of the wells in table 9.

None of the water-level measurements obtained during the withdrawal None of the water-level measurements obtained during the withdrawal test have been adjusted for natural fluctuations. These fluctuations, test have been adjusted for natural fluctuations. These fluctuations, which have to be considered in preparing the data for hydraulic analysis, which have to be considered in preparing the data for hydraulic analysis, are caused by tidal changes in the Gulf of Mexico, periodic dilation of are caused by tidal changes in the Gulf of Mexico, periodic dilation of
the rock column caused by earth tides, barometric pressure changes, and regional ground-water trends. regional ground-water trends.

Hydrographs of wells A2, B1, and B2, which include the withdrawal Hydrographs of wells A2, Bl, and B2, which include the withdrawal test period, are shown on figure 5 and those of wells B3, B4, and B5 are shown on figure 6. shown on figure 6.

Geophysical Logs Geophysical Logs

Table 10 lists the geophysical logs run in well E2 and table 11 Table 10 lists the geophysical logs run in well E2 and table 11 lists those run in well Al. Lithologic, caliper, and flowmeter logs for well Al are shown on figure 7. For the upper part of well E2, lithologic, lists those run in well Al. Lithologic, caliper, and flowmeter logs for well Al are shown on figure 7. For the upper part of well E2, lithologic, caliper, single point resistance, flowmeter, pumping temperature, and spe-caliper, single point resistance, flowmeter, pumping temperature, and specific capacity logs are shown on figure 8, and for the lower part, litho-cific capacity logs are shown on figure 8, and for the lower part, lithologic, caliper, deep induction, static and pumping temperature, and speci-logic, caliper, deep induction, static and pumping temperature, and specific capacity logs are shown on figure 9. fic capacity logs are shown on figure 9.

Chemical Analyses of Water from Selected Wells Chemical Analyses of Water from Selected Wells

Water samples from wells at the test site were collected and analy-Water samples from wells at the test site were collected and analyzed after all wells were constructed and again during the withdrawal test. zed after all wells were constructed and again during the withdrawal test, Tables 12 through 14 show the results of these analyses. Tables 12 through 14 show the results of these analyses.

Figure 5.--Hydrographs of wells A2, B1, and B2, 1976-77.

Figure 6.--Hydrographs of wells B3, B4, and B5, 1976-77.

 $\ddot{}$

Figure 7.--Caliper and flowmeter logs of well Al.

 $\sigma_{\rm{max}}$

Figure 8.--Lithologic and geophysical logs of well E2, from 160 to 540 feet.

Figure 9.--Lithologic and geophysical logs of well E2, from 460 to 1,290 feet.

Concentration of chloride in water from wells at the site increase Concentration of chloride in water from wells at the site increase with depth. For example, from 192 to 340 ft the chloride concentration with depth. For example, from 192 to 340 ft the chloride concentration
ranged from 150 to 680 mg/L (milligrams per liter), and from 500 to 1,267 ft it ranged from 16,000 to 20,000 mg/L. ft it ranged from 16,000 to 20,000 mg/L.

The following water-quality sampling procedures were used at the wells. Each water sample was collected either with a centrifugal or a turbine pump. Before collecting the initial pumped water sample, the quantity of water equal to the total volume within the well was removed at least one time. Specific gravity, specific conductance, and temperature measurements of the discharging water were then made. Only after the specific gravity or specific conductance and temperature became con-the specific gravity or specific conductance and temperature became constant during pumping were the initial water samples for chemical analysis stant during pumping were the initial water samples for chemical analysis collected. All subsequent samples were collected after the total volume collected. All subsequent samples were collected after the total volume of each well was removed. of each well was removed. The following water-quality sampling procedures were used at the
wells. Each water sample was collected either with a centrifugal or a
turbine pump. Before collecting the initial pumped water sample, the
quantity of water

Freshwater was introduced into the observation wells during drill-Freshwater was introduced into the observation wells during drilling and construction. After placement of cement in wells Bl - B5, fresh-ing and construction. After placement of cement in wells Bl - B5, freshwater was used to wash cement from the work pipe. The disposition of this water was used to wash cement from the work pipe. The disposition of this water and how it blended with the native saline water is unknown. water and how it blended with the native saline water is unknown.

WATER QUALITY NEAR THE INJECTION TEST SITE WATER QUALITY NEAR THE INJECTION TEST SITE

Water from 11 wells, other than those described earlier and ranging Water from 11 wells, other than those described earlier and ranging in depth from 45 to 400 ft near the test injection site, was collected and analyzed to provide background quality data prior to any long-term in depth from 45 to 400 ft near the test injection site, was collected and analyzed to provide background quality data prior to any long-term injection at the test site. Location of the wells is shown on figure 10 injection at the test site. Location of the wells is shown on figure 10 and construction details are given in table 15. Analyses of water from the wells are given in tables 16 and 17. Chloride concentration of water from these wells ranged from 72 to 1,100 mg/L. and construction details are given in table 15. Analyses of water from the wells are given in tables 16 and 17. Chloride concentration of water from these wells ranged from 72 to 1,100 mg/L.

Figure 10.--Location of selected wells near the Bear Creek test site.

- Applin, Paul L., 1951, Preliminary report on buried pre-Mesozoic rocks Applin, Paul L., 1951, Preliminary report on buried pre-Mesozoic rocks
in Florida and adjacent states: U.S. Geological Survey Circular 91, p. 28. 91, p. 28.
- Black, Crow and Eidsness, Inc., 1970, Water resources investigations Black, Crow and Eidsness, Inc., 1970, Water resources investigations for the Pinellas County water system, Pinellas County, Florida.

 1974, Results of drilling and testing of the test storm water injec-____ 1974, Results of drilling and testing of the test storm water injection well for the City of St. Petersburg, Florida. tion well for the City of St. Petersburg, Florida.

- Briley, Wild and Associates, Inc., and Seaburn and Robertson, Inc., 1977, Briley, Wild and Associates, Inc., and Seaburn and Robertson, Inc., 1977, Effluent disposal well investigation, McKay Creek waste water treat-Effluent disposal well investigation, McKay Creek waste water treatment facility, Pinellas County, Florida. ment facility, Pinellas County, Florida.
- Cherry, R. N., Stewart, J. W., and Mann, J. A., 1970, General hydrology Cherry, R. N., Stewart, J. W., and Mann, J. A., 1970, General hydrology of the Middle Gulf Area, Florida: Florida Geological Survey Report of the Middle Gulf Area, Florida: Florida Geological Survey Report of Investigation 56, p. 96. of Investigation 56, p. 96.
- Chen, Chih Shan, 1965, The regional lithostratigraphic analysis of Palo-Chen, Chih Shan, 1965, The regional lithostratigraphic analysis of Palocene and Eocene rocks of Florida: Florida Geological Survey Bulletin
45, p. 105. 45, p. 105.
- Greenleaf and Telesca, Inc., 1975, Construction of test wells, South Greenleaf and Telesca, Inc., 1975, Construction of test wells, South Cross Bayou Pollution Control Facility, Pinellas County, Florida. Cross Bayou Pollution Control Facility, Pinellas County, Florida.
- Hantush, M. S., 1964, Hydraulics of wells: <u>in</u> Advances of Hydroscience, vol. 1, edited by Ven Te Chow, Academic Press, New York and London, vol. 1, edited by Ven Te Chow, Academic Press, New York and London, p. 280-432. p. 280-432.
- Heath, Ralph C., and Smith, Peter C., 1954, Ground-water resources of Pinellas County, Florida: Florida Geological Survey Report of Investigation 12, p. 139. Heath, Ralph C., and Smith, Peter C., 1954, Ground-water resources of Pinellas County, Florida: Florida Geological Survey Report of Investigation 12, p. 139.
- Hickey, J. J., 1977, Hydrogeologic data for the McKay Creek subsurface-Hickey, J. J., 1977, Hydrogeologic data for the McKay Creek subsurfaceinjection test site, Pinellas County, Florida: U.S. Geological injection test site, Pinellas County, Florida: U.S. Geological Survey Open-File Report 77-802, p. 62. Survey Open-File Report 77-802, p. 62.
- Jacob, C. E., 1947, Drawdown test to determine effective radius of arte-Jacob, C. E., 1947, Drawdown test to determine effective radius of artesian wells: American Society of Civil Engineers Transactions, v. 112, sian wells: American Society of Civil Engineers Transactions, v. 112, p. 1047-1070. p. 1047-1070.
- Kaufman, M. I., 1973, Subsurface waste water injection, Florida: American Kaufman, M. I., 1973, Subsurface waste water injection, Florida: American Society of Civil Engineers Proceedings, Journal of Irrigation and Society of Civil Engineers Proceedings, Journal of Irrigation and Drainage Division, v. 99, no. IT1, p. 53-70. Drainage Division, v. 99, no. IT1, p. 53-70.
- Klinkenberg, L. J., 1941, The permeability of porous media to liquids and Klinkenberg, L. J., 1941, The permeability of porous media to liquids and gases: American Petroleum Institute Drilling Production Practices, gases: American Petroleum Institute Drilling Production Practices, p. 200-213. p. 200-213.

 ^{1978,} Drilling and testing of the monitoring and injection wells at ____ 1978, Drilling and testing of the monitoring and injection wells at the Southwest Wastewater treatment plant for the City of St. Peters-the Southwest Wastewater treatment plant for the City of St. Petersburg, Florida. burg, Florida.

- Lohman, S. W. and others, 1972, Definitions of selected ground-water terms Lohman, S. W. and others, 1972, Definitions of selected ground-water terms - revisions and conceptual refinements: U.S. Geological Survey Water-- revisions and conceptual refinements: U.S. Geological Survey Water-Supply Paper 1988, p. 21. Supply Paper 1988, p. 21.
- Puri, Harbans, S., Faulkner, Glen L., Winston, George 0., 1973, Hydrogeolo-Puri, Harbans, S., Faulkner, Glen L., Winston, George 0., 1973, Hydrogeology of subsurface liquid-waste storage in Florida, preprints of papers presented at the Second International Symposium on Underground Waste gy of subsurface liquid-waste storage in Florida, preprints of papers presented at the Second International Symposium on Underground Waste Management and Artificial Recharge, New Orleans, Louisiana, Sept. 26-30, Management and Artificial Recharge, New Orleans, Louisiana, Sept. 26-30, vol. 2: American Association of Petroleum Geologists, Inc., Tulsa, vol. 2: American Association of Petroleum Geologists, Inc., Tulsa, Oklahoma, p. 825-841. Oklahoma, p. 825-841.
- Rosenshein, J. S. and Hickey, J. J., 1977, Storage of treated sewage efflu-Rosenshein, J. S. and Hickey, J. J., 1977, Storage of treated sewage effluent and storm water in a saline aquifer, Pinellas Peninsula, Florida: ent and storm water in a saline aquifer, Pinellas Peninsula, Florida: Ground Water, vol. 15, no. 4, p. 289-293. Ground Water, vol. 15, no. 4, p. 289-293.
- Sinclair, William C., 1974, Hydrogeologic characteristics of the surficial Sinclair, William C., 1974, Hydrogeologic characteristics of the surficial aquifer in northwest Hillsborough County, Florida: Florida Bureau of aquifer in northwest Hillsborough County, Florida: Florida Bureau of Geology Information Circular 86, p. 98. Geology Information Circular 86, p. 98.
- Teeuw, D., 1971, Prediction of formation compaction from laboratory com-Teeuw, D., 1971, Prediction of formation compaction from laboratory compressibility data: American Institute of Mining Metallurgists pressibility data: American Institute of Mining Metallurgists Petroleum Engineers Transactions, v. 251, p. 263-271. Petroleum Engineers Transactions, v. 251, p. 263-271.
- Vernon, R. 0., 1970, The beneficial use of zones of high transmissivities Vernon, R. 0., 1970, The beneficial use of zones of high transmissivities in Florida subsurface for water storage and waste disposal: Florida in Florida subsurface for water storage and waste disposal: Florida Bureau of Geology Information Circular 70, p. 39. Bureau of Geology Information Circular 70, p. 39.

Table 1.--Record of wells at the Bear Creek test injection site

[Location of wells is shown on figure 2.] [Location of wells is shown on figure 2.]

 $1/\sqrt{\omega}$ is unnumbered because no data were obtained concerning hydrologic characteristics.

 $\rm 21$

Table 2.--Lithologic logeof well El Table 2.--Lithologic log of well El

 $\ddot{}$

Table 2.--<u>Lithologic log of well El</u> - continued

 $\ddot{\cdot}$

,

 ~ 20 \mathcal{L}_{max}

 \mathcal{F}

 $\hat{\mathcal{A}}$

 $\frac{1}{2}$

 $\Delta \sim 1$

 $\begin{pmatrix} 1 & 1 \\ 1 & 1 \\ 1 & 1 \end{pmatrix}$ $\sim 10^{-4}$

 $\left| \begin{matrix} 1 \\ 1 \\ 1 \end{matrix} \right|$

 $\ddot{}$

 $\ddot{}$ $\hat{\mathcal{E}}$

Table 2.--<u>Lithologic log of well El</u> - continued

 \sim 100 μ

 \sim \sim

 \sim

 \bullet

 $\ddot{}$

 $\frac{1}{2}$

 $\ddot{\cdot}$

 \sim \mathcal{L}_{max}

Table 4.--Vertical intrinsic permeability of cores from well E2

[Analyses performed by Core Laboratories, Inc., Dallas, Texas. Nitrogen [Analyses performed by Core Laboratories, Inc., Dallas, Texas. Nitrogen gas intrinsic permeabilities corrected for Klinkenberg effect (Klinkenberg, gas intrinsic permeabilities corrected for Klinkenberg effect (Klinkenberg, 1941) by Core Laboratories. Kinematic viscosity of the formation and dis-1941) by Core Laboratories. Kinematic viscosity of the formation and distilled waters used in the tests were 0.960 centistokes and 0.955 centi-tilled waters used in the tests were 0.960 centistokes and 0.955 centistokes, respectively. Kinematic viscosity of the nitrogen gas was 15.391 stokes, respectively. Kinematic viscosity of the nitrogen gas was 15.391 centistokes. Temperature of all of the fluids was 75°F. Intrinsic per-centistokes. Temperature of all of the fluids was 75°F. Intrinsic permeabilities may be converted to hydraulic conductivity as shown in Lohman meabilities may be converted to hydraulic conductivity as shown in Lohman and others (1972, p. 10).] and others (1972, p. 10).]

Table 5.--Porosity and interval transit time of cores from well E2

 \mathcal{A} $\ddot{}$

> [Analyses performed by Core Laboratories, Inc., Dallas, Texas. Porosity determined at zero gage pressure. Interval transit time for ζ 40-741 feet and 760-761 feet cores determined at 200 lb/in⁻ and 750 lb/in⁻ effective overburden pressure, respectively. Interval transit time for all other cores determined at 1,000 lb/in effective overburden pressure. Effec-overburden pressure, respectively. Interval transit time for all other cores determined at 1,000 Ib/in effective overburden pressure. Effective overburden pressure is the external pressure minus internal pressure tive overburden pressure is the external pressure minus internal pressure exerted on core.] exerted on core.] [Analyses performed by Core Laboratories, Inc., Dallas, Texas. Porosity determined at zero gage pressure. Interval transit time for 740-741 feet and 760-761 feet cores determined at 200 Ib/in and 750 Ib/in effective

Table 6.--Compressibility of cores from well E2

[Analyses performed by Core Laboratories, Inc., Dallas, Texas. Core Laboratories calculated pore volume compressibility, which is change in [Analyses performed by Core Laboratories, Inc., Dallas, Texas. Core Laboratories calculated pore volume compressibility, which is change in pore volume divided by average pore volume divided by initial bulk vol-pore volume divided by average pore volume divided by initial bulk volume divided by pressure change. Compressibility was calculated from the Core Laboratory results by multiplying pore volume compressibility by average pore volume divided by initial bulk volume. Average pore volume was calculated from values measured over a selected range of pressures. Initial bulk volume was measured at the first pressure in the selected Initial bulk volume was measured at the first pressure in the selected ume divided by pressure change. Compressibility was calculated from the
Core Laboratory results by multiplying pore volume compressibility by
average pore volume divided by initial bulk volume. Average pore volume
was calc

pressure range.] pressure range.]

Date pumping began	Depth interval (f _t)	Elapsed time (min)	Discharge (ga1/min)	Specific capacity [(gal/min)/ft] of drawdown]	Comments
11/6/74	$192 - 250$	30	150	13	
11/14/74	$192 - 350$	30	265	241	
11/18/74	$192 - 503$	30	260	228	
12/13/74	$500 - 665$	30	75	66	Drill rod and bit in hole at 665 feet
1/21/75	$500 - 865$	30	145	91	Drill rod and bit in hole at 862 feet
8/29/75	500-1290	90	226	461	Drill rod and bit out of hole

Table 7.--Specific-capacity data for well E2

Contractor

and the state of the state

 $\mathcal{F}(\mathcal{L})$

 $\mathcal{L}^{(1)}$. $\overline{}$

 $\overline{}$

Table 8.--Water levels in well A2 during well A1 withdrawal test

 \bullet .

 Δ

 $\frac{1}{2}$

[Test began 5-18-76 at 1545 and ended 5-21-76 at 1630. Discharge of well Al was 3,450 gallons per minute.] [Test began 5-18-76 at 1545 and ended 5-21-76 at 1630. Discharge of well Al was 3,450 gallons per minute.]

 \cdot

 ~ 1 .

 $\ddot{\cdot}$

r

Table 9.--Water levels in wells B1, B2, B3, B4, and B5 during well Al withdrawal test - continued

 $\frac{1}{\sqrt{2}}$

 $\mathcal{L}^{\mathcal{L}}$

 $[Test$ began 5-18-76 at 1545 and ended 5-21-76 at 1630. Discharge of well Al was 3,450 gal/min. Water levels are in feet below mean sea level.] Al was 3,450 gal/min. Water levels are in feet below mean sea level.]

Table 10.--Summary of geophysical logs of well E2

 \sim

 $\frac{1}{2}$

 $\frac{1}{\sqrt{2}}$

[Logs are on file in the U.S. Geological Survey Southwest Florida Subdistrict office.] [Logs are on file in the U.S. Geological Survey Southwest Florida Subdistrict office.]

Table 11.--Summary of geophysical logs of well Al

 \bullet .

 $\bar{\bar{z}}$

 $\ddot{\cdot}$

[Logs are on file in the U.S. Geological Survey Southwest Florida Subdistrict office.] [Logs are on file in the U.S. Geological Survey Southwest Florida Subdistrict office.]

Table 12.--Chemical analyses of water from selected wells at the test site

and the state of

 $\mathcal{F}_{\mathcal{A}}$.

 \mathcal{L}^{\pm} $\overline{}$

 $\mathcal{L}^{(1)}$ and

 $\mathcal{L}^{(1)}$

and the contract of the contract of

Table 12.--Chemical analyses of water from selected wells at the test site - continued

the company's company's

Table 12.--Chemical analyses of water from selected wells at the test site - continued

л.

 $\mathcal{F}(\mathcal{A})$

 \sim $\ddot{}$

 $\ddot{\mathbf{t}}$

Table 12.--Chemical analyses of water from selected wells at the test site - continued

 ~ 100 \mathbf{r}

 $\mathcal{L}^{(k)}$

 42

and the company of the com-

Table 13.--Concentrations of trace elements in water from selected wells at the test site

 $\mathcal{F}^{\text{max}}_{\text{max}}$

 \bullet l,

Table 13.--Concentrations of trace elements in water from selected wells at the test site - continued

 ~ 10 km

Contractor

 $\mathbf{z} = \mathbf{z}^{\mathbf{z}}$

 $\mathcal{L}(\mathcal{F})$

Table 14.--Concentrations of nitrogen, phosphorus, coliform, and streptococci from selected wells at the test site

Contract Contract

 $\label{eq:2.1} \frac{1}{2} \int_{\mathbb{R}^3} \frac{1}{\sqrt{2\pi}} \left(\frac{1}{\sqrt{2\pi}} \right)^2 \frac{1}{\sqrt{2\pi}} \, \frac{1}{\sqrt{2\pi}} \int_{\mathbb{R}^3} \frac{1}{\sqrt{2\pi}} \frac{1}{\sqrt{2\pi}} \, \frac{1}{\sqrt{2\pi}} \$

 $\mathcal{F}_{\mathcal{A}}$.

 $\ddot{}$. $\ddot{}$

 \sim

 \mathcal{A}

 \mathcal{A} is.

 $\mathcal{L}^{(k)}$

the control of the con-

Table 15.--Record of wells near the Bear Creek test site

 $\mathcal{F}_{\text{c},\text{c}}$

 $\mathbf{L}^{(1)}$ $\ddot{}$

[Location of wells is shown in figure 10.] [Location of wells is shown in figure 10.]

 \mathfrak{t}^{\prime}

Table 16.--Chemicai analyses of water from selected wells near the test site

 $\langle \rangle$ \ddotsc

 $\mathcal{A}^{\mathcal{I}}$

 $\sim 10^{-11}$

Contract

Table 16.--Chemical analyses of water from selected wells near the test site - continued

 $\frac{d}{dt} \frac{d}{dt} \frac{d\phi}{dt}$

 ω \bullet

 $\overline{6}$

 \sim

Table 16.--Chemical analyses of water from selected wells near the test site - continued

 ~ 100 a.

 $\frac{1}{\Delta}$ \mathbf{a}_1 . \mathbf{a}_2

Table 16.--Chemical analyses of water from selected wells near the test site - continued

 $\frac{1}{2} \frac{1}{2} \frac{1}{2} \frac{1}{2}$

 $\sigma_{\rm{max}}$

 51

 \sim

Table 17.--Concentrations of nitrogen and phosphorus in water from selected wells near the test site

 \mathbb{R}^2

 \mathcal{A}

 $\Delta \phi$ \sim

the committee of the

 \mathcal{L}_{max}

 \bullet . $\ddot{}$

ပ္မ